

FOOTING SCHEDULE						
MARK	SIZE	REINFORCING	REMARKS			
WF1	2'-6"x 12"x CONT.	3#5 CONT. WITH #5 AT 12" o/c BOT.				
F30	3'-0"x 3'-0"x 12"	4#4 E.W. BOT.				
FOOTING SCHEDULE NOTES:						
1. FOOTINGS HAVE BEEN DESIGNED USING AN ALLOWABLE BEARING PRESSURE						

AVE BEEN DESIGNED USING A OF 2000 PSF. THIS VALUE SHALL BE FIELD VERIFIED PER PROJECT INSPECTION

REQUIREMENTS BY A MARYLAND REGISTERED GEOTECHNICAL ENGINEER. 2. THE CONCRETE COMPRESSIVE STRENGTH (fc) FOR ALL FOOTINGS SHALL BE

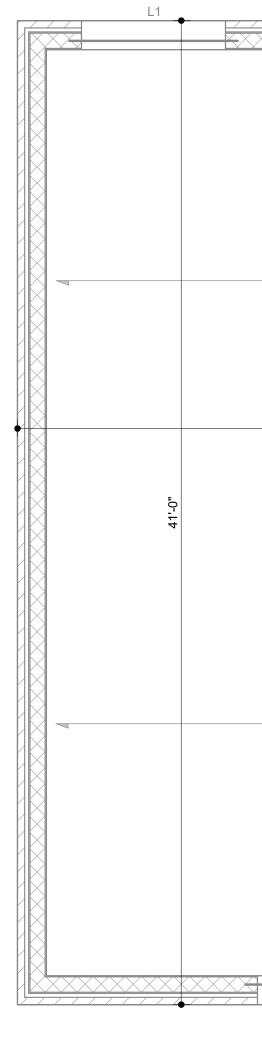
3000 PSI.

SCALE: 1/4" = 1'-0"

1 5 5 8 8

 4
 5

- 2. STRUCTURAL SLAB SHALL BE 5" CONCRETE SLAB ON GRADE (fc = 3000 PSI) REINFORCED WITH 6" x 6" - W2.9 / W2.9 WELDED WIRE FABRIC POURED OVER VAPOR
- 4. ASSUMED SOIL BEARING VALUE = 2000 PSF WAS USED IN DESIGN OF THE STRUCTURE. THIS VALUE SHALL BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER PRIOR TO THE INSTALLATION OF ANY FOUNDATIONS. CONTRACTOR SHALL SUBMIT GEOTECHNICAL REPORT PRIOR TO THE ONSET OF CONSTRUCTION.
- 6. PRE-ENGINEERED METAL STAIRS AND LANDINGS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS TO ENGINEER FOR REVIEW. SEE ARCHITECTURAL DRAWINGS FOR RISER AND TREAD DIMENSIONS AND INTERFACE AT BUILDING EXTERIOR. STAIR AND LANDING FRAMING CONNECTIONS AND SUPPORT AT GRADE ARE PART OF THE DELEGATED DESIGN AND SHALL BE INCLUDED IN THE CONTRACTORS SUBMITTAL. POST FOUNDATIONS SHALL BE A MINIMUM OF 16" DIAMETER SONOTUBES REINFORCED WITH 5#6 VERTICAL BARS AND #3 TIES AT 12"o/c. A SONOTUBE OR TURNDOWN SLAB SHALL BE PROVIDED TO SUPPORT THE STAIR STRINGERS AT GRADE. STAIRS AND LANDINGS SHALL BE DESIGNED TO BE DISASSEMBLED FOR FUTURE BUILDING EXPANSION, USE BOLTED CONNECTIONS WHERE POSSIBLE.
- 7. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL AND SITE DRAWINGS PRIOR TO CONSTRUCTION.
- 8. PROVIDE FOOTING IN SLAB ON GRADE AT ALL NON-LOAD BEARING BLOCK PARTITIONS PER DETAIL 12 ON S202. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF ANY MASONRY PARTITIONS NOT SPECIFICALLY SHOWN ON STRUCTURAL DRAWINGS.
- 9. PROVIDE SLAB DEPRESSIONS AT ALL ELECTRICAL FLOORING BOXES AND RECESSED RACEWAYS IN SLAB ON GRADE. REFER TO DETAILS 9 AND 10 ON S202 FOR ADDITIONAL INFORMATION. REFER TO ELECTRICAL DRAWINGS FOR EXACT LOCATIONS.
- 10. PROVIDE EQUIPMENT PADS WHERE INDICATED ON MEP PLANS. REFER TO DETAIL 8 ON S202 FOR ADDITIONAL INFORMATION.
- 11.LOCATION OF CONCRETE ENCASED CONDUITS ENTERING THE BUILDING ARE SHOWN ON PLAN BASED ON MEP DRAWINGS, COORDINATE EXACT SIZE AND LOCATION WITH MEP PRIOR TO INSTALLATION. IF CONDITIONS DIFFER FROM WHAT IS SHOWN ON THE FOUNDATION PLAN CONTACT MORABITO CONSULTANTS FOR ADDITIONAL REVIEW.

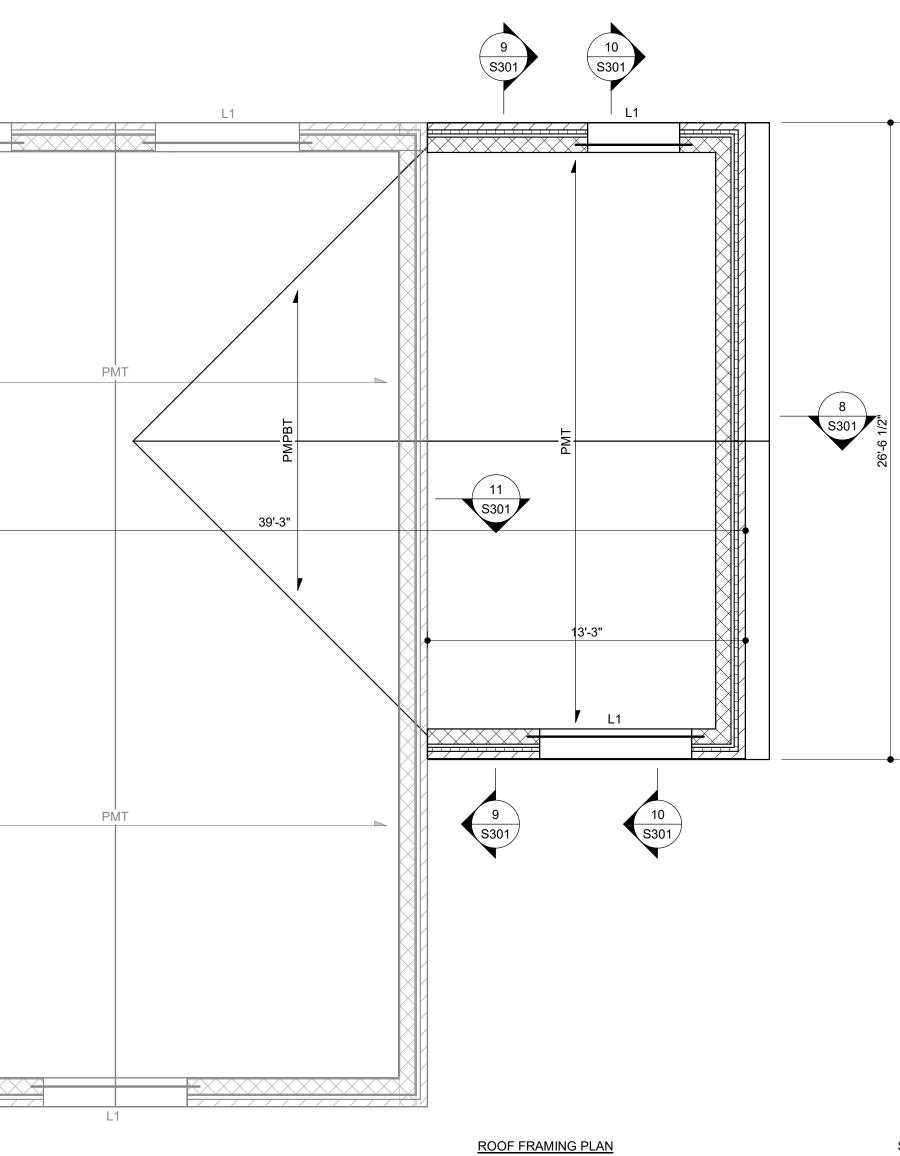


	LINTEL SCHEDULE					
MARK	MEMBER	TYPE	REMARKS			
L1	PRECAST CONCRETE LINTEL	• •	8" MIN. BEARING AT EACH END			
LINTEL SC	CHEDULE NOTES:					

DESIGN SUBMISSION.

DRAWINGS.

1.
2.
3.



1. FOR EXACT SIZE AND LOCATION OF WALL OPENINGS, SEE ARCHITECTURAL

2. PRECAST LINTEL DESIGN SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER AND SUBMITTED TO THE ENGINEER OF RECORD FOR APPROVAL. ALL REQUIRED CONNECTIONS OF THE PRECAST TO THE STRUCTURE ARE CONSIDERED TO BE A PART OF THE PRECAST LINTEL

3. ALL STRUCTURAL STEEL IN CONTACT WITH MASONRY SHALL BE GALVANIZED IN ACCORDANCE WITH SPECIFICATIONS.

4. FILL ALL MASONRY JAMBS SOLID EACH SIDE OF OPENING WITH 3000 PSI GROUT, SEE DETAIL 11 ON S203 FOR ADDITIONAL INFORMATION. FOR TYPICAL JAMB REINFORCING REQUIREMENTS SEE DETAILS ON S203.

MASONRY WALL SCHEDULE				
MARK	EXTERIOR MASONRY WALLS			
BLOCK SIZE	8" CMU			
1 TO ROOF	#6 AT 24"o/c			

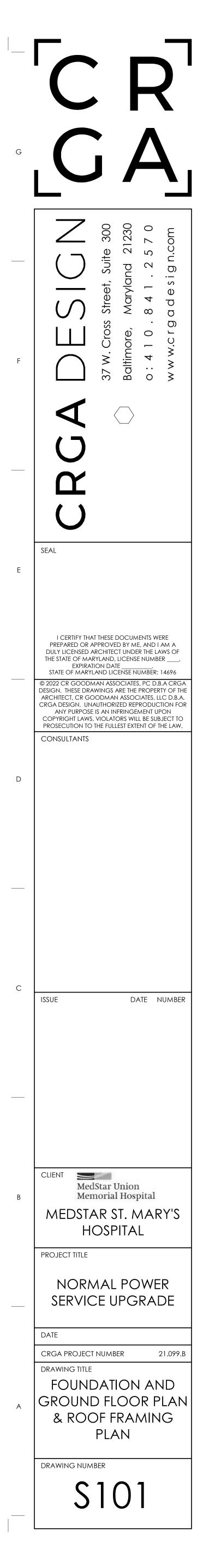
. ALL MASONRY REINFORCING SHALL HAVE THE LAP SPLICES PER SCHEDULE

. ALL REINFORCED MASONRY SHALL BE FILLED 100% SOLID WITH 3000 PSI GROUT. . PROVIDE ADDITIONAL VERTICAL BARS AT END OF WALL, CORNERS, AND JAMBS PER

DETAILS 1 THROUGH 6 ON S203. BAR SIZE SHALL MATCH SIZE INDICATED IN MASONRY WALL SCHEDULE.

SCALE: 1/4" = 1'-0"

- 1. ELEVATION OF ROOF TRUSS BEARING = EL. <u>102.50 (102.17 per RFI from previous project)</u> 2. STRUCTURAL ROOF SHALL BE 1 1/2" x 22 GAUGE TYPE 'B' GALVANIZED METAL DECK SUPPORTED BY PRE-ENGINEERED METAL STUD ROOF TRUSSES AT 4'-0" o/c MAX. SEE S301 FOR DECK FASTENING REQUIREMENTS.
- 3. PRE-ENGINEERED METAL STUD ROOF TRUSSES ARE NOTED ON PLAN THUS: PMT = PRE-ENGINEERED METAL ROOF TRUSS
- PMPBT = PRE-ENGINEERED METAL PIGGYBACK TRUSS 4. REFER TO ARCHITECTURAL DRAWINGS FOR WORKING POINTS AND PITCHES OF PRE-
- ENGINEERED METAL STUD TRUSSES. 5. FOR ROOF PROFILE AND LOCATION OF RIDGE, REFER TO ARCHITECTURAL DRAWINGS.
- 6. WALL TYPES INDICATED ON PLAN THUS: REINFORCED MASONRY BLOCK WALLS (ASTM C-90) WITH BRICK FACADE, SEE SCHEDULE ON S101 FOR REINFORCING
- 7. PROVIDE 8" WIDE x 8" DEEP CONT. BOND BEAM AROUND THE PERIMETER OF THE BUILDING WITH 2#5 CONT. BOTTOM REINFORCING AND FILLED SOLID WITH 3000 PSI GROUT AT ROOF TRUSS BEARING. PROVIDE 90 DEGREE HOOK BARS AT CORNERS.
- 8. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL AND SITE DRAWINGS PRIOR TO CONSTRUCTION.
- 9. FOR LOCATIONS AND SIZES FOR ANY HUNG MECHANICAL UNITS, SEE MECHANICAL DRAWINGS. COORDINATE WEIGHTS AND LOCATIONS WITH TRUSS DESIGNER.
- 10. FOR LINTEL/BEAM SCHEDULE SEE S101. ALL LINTELS SHALL BE INSTALLED AT THE TOP OF THE DOOR OR LOUVER OPENING, REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS OF THE HEAD OF OPENINGS AND LOUVER LOCATIONS.
- 11. COORDINATE ANY NEW DUCT/CONDUIT/PIPE PENETRATION SIZES AND LOCATIONS INTO EXISTING BUILDING EXTERIOR WALL WITH ARCHITECT AND MEP. EXISTING EXTERIOR WALLS ARE CMU WITH BRICK FACADE, FOR NEW OPENINGS GREATER THAN 2'-0" AND LESS THAN 5'-0" PROVIDE (1)-6"x3 1/2"x5/16" ANGLE (LLV) PER 4" WIDTH OF WALL. FOR ANY NEW OPENINGS LARGER THAN 5'-0" CONTACT THE ENGINEER OF RECORD FOR REVIEW.



1	2	

	FREQUENCY OF INSPECTIONS		REFERENCE	CRITERIA		FREQUENCY OF INSPECTIONS		REFERENCE CRITERIA		
TASK, VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	REF. STD.	IBC REF.	- TASK, VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	REF. STD.	IBC REF.	
STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL				1705.2.2	SOILS				1705.6	
1. MATERIAL VERIFICATION OF COLD FORMED STEEL MATERIAL & STEEL BAR			SJI2002		1. CONTROLLED FILL PLACED UNDER SITE PERMIT					
JOISTS					2. CONTROLLED FILL PLACED UNDER THIS BUILDING PERMIT	Х				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARD SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS		X	APPLICABLE ASTM MATERIAL STANDARDS		3. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		Х			
B. MANUFACTURER'S CERTIFIED TEST REPORTS.		Х			4. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED					
. INSPECTION OF WELDING AND SCREWED CONNECTIONS			AISI S100-07/S1-10 AISI S200-07		PROPER MATERIAL		X			
A. COLD-FORMED STEEL - FLOOR AND ROOF DECK		Х	AWS D1.3		5. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		Х			
B. COLD-FORMED METAL STUD WALLS		X	AISI S211-07		6. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	Х				
C. COLD-FORMED METAL STUD JOISTS		Х	AISI S212-07		7. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY		X			
D. COLD-FORMED STEEL TRUSSES		Х					N N			
E. STEEL BAR JOISTS		Х	SJI2002		8. VERIFY INSTALLATION OF DRAIN TILE (GRAVITY/MECHANICAL)		X			
F. REINFORCING STEEL:										
1.) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706		Х		-	GENERAL NOTES					
2.) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT	Х		AWS D1.4		CONTRACTORS RESPONSIBILITIES: 1. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DET SEQUENCE, AND TO ENSURE THE STABILITY OF THE BUILDING AND ITS COMPONENT PARTS, AND THE ADEQUACY OF TEMPORARY OR INCOMPLETE CONNECTIONS, DURING B					
3.) SHEAR REINFORCEMENT	Х		ACI 318: SECTION 3.5.2		ANY SHORING, SHEETING, TEMPORARY GUYS, BRACING, OR TIEDOWNS THAT MIGHT BE NECESSARY. SUCH MATERIAL IS NOT SHOWN ON THE DRAWINGS. IF APPLIED, THEY S					
4.) OTHER REINFORCEMENT		Х			2. CONTRACTOR SHALL VERIFY CONDITION IN THE FIELD AN	AND SHALL REMAIN THE CONTRACTOR'S PROPERTY. 2. CONTRACTOR SHALL VERIFY CONDITION IN THE FIELD AND IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER OF ANY (
6. COLD FORMED STEEL TRUSSES & STEEL BAR JOISTS.			AISI S214-07 SJI2002		RESPONSIBLE FOR THE SAME. 3. CONTRACTOR SHALL COORDINATE WITH ALL RELATED TRADES FOR DETAILING, FABRICATION AND ERECTION PRIOR TO SUBMITTING SHOP DRAWINGS FOR APPROVA					
A. VERIFY THAT TEMPORARY INSTALLATION RESTRAINT/BRACING/BRIDGING IS INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS & JOIST SUBMITTAL PACKAGE.		Х		-	 ALL STRUCTURAL WORK SHALL BE COORDINATED WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, ETC. REQUIREMENTS. DISCREPANCIES AND/OR INTERFEF ARCHITECT/ENGINEER IMMEDIATELY. THE ENGINEER HAS NO EXPERTISE IN, AND TAKES NO RESPONSIBILITY FOR, CONSTRUCTION MEANS AND METHODS OR JOBSITE SAFETY DURING CONSTRUCTION. PROCESSING AND/OR APPROVED SUBMITTALS MADE BY THE CONTRACTOR WHICH MAY CONTAIN INFORMATION RELATED TO CONSTRUCTION METHODS OF SAFETY ISSUES SUCH ISSUES MIGHT BE DISCUSSED, SHALL NOT BE CONSTRUED AS VOLUNTARY ASSUMPTION BY THE ENGINEER OF ANY RESPONSIBILITY OF EACH CONTRACTOR TO FOLL REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE ENGINEER IS NOT ENGAGED IN AND DOES NOT SUPERVISE CONSTRUCTION. 					
B. VERIFY THAT PERMANENT INSTALLATION OF INDIVIDUAL TRUSS & JOIST RESTRAINT/BRACING/BRIDGING IS INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS & JOIST SUBMITTAL PACKAGE		Х								
C. INSPECT ALL TRUSS & JOIST MEMBERS FOR BOWING, WARPING, TWISTING AND OTHER DAMAGE.		Х	SJI2002		DEMOLITION:					
D. INSPECT TRUSS & JOIST PLUMBNESS.		Х	SJI2002		1. REMOVE EXISTING CONSTRUCTION AS SHOWN ON PLANS 2. EXISTING STRUCTURAL ELEMENTS SHALL REMAIN UNLES			IO BE REMOVED.		
E. INSPECT ALL TRUSS & JOIST BOLTED & WELDED CONNECTIONS TO BUILDING SUPPORT STRUCTURE		х	AISI S212-07/S2-08 SJI2002		 IF FIELD CONDITIONS DIFFER FROM THOSE SHOWN ON DRAWINGS, NOTIFY ARCHITECT/ENGINEER BEFORE PROCEEDING WITH DEMOLITION. CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY SHORING AS REQUIRED TO ASSURE SAFE DEMOLITION PROCEDURES. 					

TMS 602

ACI 530.1

ASCE 6

ART. 1.5

ART. 1.4 B

ART. 1.5 B.1, B.3

ART. 2.1, 2.6 A

ART. 3.3 B

ART. 2.4 B, 2.4 H

ART. 3.4, 3.6 A

ART. 3.6 B

ART. 2.1 C

ART. 3.2 D, 3.2 F

ART 2.4, 3.4

ART. 3.4, 3.2 E, 3.6 A

ART. 2.6 B, 2.4 G.1.b

ART. 3.3 F

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ART. 1.8 C, 1.8 D

ART. 3.6 B

ART. 3.5, 3.6 C

ART. 3.3 B.8

ART 1.4 B.2a.3, 1.4 B.2.b.3,

1.4 B.2.c.3, 1.4 B.3, 1.4 B.4

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ART. 3.3 B

3

TASK, VERIFICATION AND INSPECTION TASK	FREQUENCY (OF INSPECTIONS	REFERENCE CRITERIA		
TASK, VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	REF. STD.	IBC REF.	
CONCRETE CONSTRUCTION			ACI 318	1705.3	
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT		x	ACI 318: 2.4, 7.1-7.7	1910.4	
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1705.2.2, ITEM 2 (IBC)			AWS D1.4 ACI 318: 3.5.2		
3. INSPECTION OF ANCHORS CAST IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED		X	ACI 318: 8.1.3, 21.2.8	1908.5, 1909.1	
4. INSPECTION OF ANCHORS POST INSTALLED IN HARDENED CONCRETE.		Х	ACI 318: 3.8.6, 8.1.3, 21.2.8	1909.1	
5. VERIFY USE OF REQUIRED DESIGN MIX.		Х	ACI 318: CH. 4, 5.2-5.4	1904.2, 1910.2, 1910.3	
6. AT THIS TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF CONCRETE	Х		ASTM C172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10	
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATIONS TECHNIQUES	Х		ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8	
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		x	ACI 318: 5.11, 5.13	1910.9	
9. INSPECTION OF PRESTRESSED CONCRETE:					
A. APPLICATION OF PRESTRESSING FORCES.	Х				
B. GROUTING OF BOUNDED PRESTRESSING TENDONS IN THE SEISMIC FORCE-RESISTING SYSTEM	Х		ACI 318: 18.18.4, 18.20		
10.ERECTION OF PRECAST CONCRETE MEMBERS.		Х	ACI 318: CH. 16		
11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS		Х	ACI 318: 6.2		
12.INSPECT FORMWORK FOR SHAPE LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		Х	ACI 318: 6.1.1		

	FREQUENCY	OF INSPECTIONS	REFERENCE CRITERIA		
TASK, VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	IBC REF.	TMS 402 ACI 530 ASCE 5	
MASONRY CONSTRUCTION LEVEL B			1705.4		
1. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED		х			
2. VERIFICATION OF <i>f'm</i> AND <i>f'aac</i> PRIOR TO CONSTRUCTION EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE		x			
3. VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATING GROUT	Х				
4. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:					
A. PROPORTIONS OF SITE-PREPARED MORTAR.		Х			
B. CONSTRUCTION OF MORTAR JOINTS		Х			
C. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES		Х			
D. LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES		X			
E. PRESTRESSING TECHNIQUE		Х			
F. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X (FIRST 5000 SQ. FT.)	X (AFTER FIRST 5000 SQ. FT.)			
5. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE					
A. GROUT SPACE IS CLEAN		X			
B. GRADE, TYPE AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS AND PRESTRESSING TENDONS AND ANCHORAGES		Х		SEC. 1.16	
C. PLACEMENT OF REINFORCEMENT AND CONNECTORS AND PRESTRESSING TENDON AND ANCHORAGES		X		SEC. 1.16	
D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS		x			
E. CONSTRUCTION OF MORTAR JOINTS		X			
6. DURING CONSTRUCTION THE FOLLOWING SHALL BE VERIFIED:					
A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X			
B. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION		х		SEC. 1.16.4.3, 1.17.1	
C. WELDING OF REINFORCING BARS	Х			SEC. 2.1.8.7.2, 3.3.3.4(c), 8.3.3.4(b)	
D. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)		x	SEC. 2104.3, 2104.4		
E. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	Х				
F. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	Х				
G. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	X (FIRST 5000 SQ. FT.)	X (AFTER FIRST 5000 SQ. FT.)			
7. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS		х	SEC. 2105.2.2, 2105.3		/
8. VERTICAL MASONRY FOUNDATION ELEMENTS		Х	1705.4		

SPECIAL INSPECTIONS NOTES:

1. THE OWNER WILL ENGAGE (SEE CONTRACT REQUIREMENTS) THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON WORK INDICATED IN THE SCHEDULE OF SPECIAL INSPECTIONS. IN ACCORDANCE WITH THE PROVISIONS OF CHAPTER 17 OF THE INTERNATIONAL BUILDING

CODE. 2. SPECIAL INSPECTIONS AND TESTING SHALL BE PERFORMED ON A CONTINUOUS OR PERIODIC FREQUENCY AS

NOTED IN THE SCHEDULE. 3. REFER TO THE GENERAL NOTES AND SPECIFICATIONS FOR ADDITIONAL INSPECTION AND TESTING REQUIREMENTS.

4. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF

DISCREPANCIES ARE NOT CORRECTED, CONTACT MORABITO CONSULTANTS PRIOR TO COMPLETION OF THAT PHASE OF THE WORK.

5. THE SPECIAL INSPECTOR SHALL SUBMIT INSPECTION REPORTS TO THE CONTRACTOR, ARCHITECT, OWNER AND MORABITO CONSULTANTS. REPORTS SHALL DOCUMENT REQUIRED INSPECTIONS AND CORRECTIONS OF ANY DISCREPANCIES. REPORTS SHALL BE PROVIDED AT INTERVALS CONVEYING THE PROGRESS OF CONSTRUCTION.

- RMINE ERECTION PROCEDURES AND ECTING. THIS INCLUDES THE ADDITION OF ALL BE REMOVED AS CONDITIONS PERMIT
- SUREMENTS AS REQUIRED AND BE
- CES SHALL BE REPORTED TO THE
- R PARTICIPATION IN MEETINGS WHERE ALL APPLICABLE SAFETY CODES AND

- 1. ALL WALLS TO BE UNDERPINNED SHALL BE COMPLETED IN 4'-0" WIDE SECTIONS MAXIMUM. 2. THE UNDERPINNING SHALL BE DESIGNED BY AN EXPERIENCED REGISTERED STRUCTURAL ENGINEER WHO SHALL SUBMIT CALCULATIONS, DETAILS, AND CONSTRUCTION PROCEDURES FOR APPROVAL PRIOR TO
- COMMENCEMENT OF WORK. 3. THE DETAILS ON THE PLANS ARE CONCEPTUAL, AND MUST BE CONFIRMED BY THE UNDERPINNING ENGINEER.
- 4. THE EXCAVATION SHALL BE SHORED, AS NECESSARY. 5. NEW CONCRETE SHALL HAVE A SHRINKAGE COMPENSATING ADMIXTURE, AND BE POURED THE FULL WIDTH OF THE EXISTING FOOTING. 6. ADJACENT SECTIONS SHALL OBTAIN FULL DESIGN STRENGTH OF 3000 PSI PRIOR TO EXCAVATION OF ADJACENT SECTIONS.
- 7. ONLY PERSONNEL EXPERIENCED WITH CONCRETE UNDERPINNING SHALL COMPLETE ALL WORK.
- CONTROLLED FILL AND BACKFILL: 1. SAMPLES OF ALL MATERIALS THAT THE CONTRACTOR PROPOSES TO USE FOR COMPACTED FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. 2. SAMPLES OF ALL MATERIALS THAT THE CONTRACTOR PROPOSES TO USE FOR COMPACTED FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. 3. SAMPLES OF ALL MATERIALS THAT THE CONTRACTOR PROPOSES TO USE FOR COMPACTED FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. 3. SAMPLES OF ALL MATERIALS THAT THE CONTRACTOR PROPOSES TO USE FOR COMPACTED FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. 5. SAMPLES OF ALL MATERIALS THAT THE CONTRACTOR PROPOSES TO USE FOR COMPACTED FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. 2. COMPACTED FILL SHALL CONSIST OF LOCAL MATERIAL FREE OF DELETERIOUS MATTER AND CLASSIFIED SP, SW, SM, SC, GP, GW, GM, OR GC PER ASTM D 2487.
- 3. THE CONTROL OF THE MOISTURE FOR PLACING THE FILL WILL BE BASED ON THE RESULTS OF COMPACTION TESTS PER ASTM D-1557. 4. ALL COMPACTED FILL SHALL HAVE A DENSITY OF AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557. 5. PRIOR TO PLACEMENT OF ANY FILLS, THE SITE SHALL BE STRIPPED OF ALL TOPSOIL, VEGETATION, ROCKS, AND ORGANIC MATERIALS AND THE EXPOSED SUBGRADE SHALL BE COMPACTED IN PLACE TO A CONFIRMED
- DENSITY OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY. 6. FILL MATERIAL SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 6" IN THICKNESS AND SHALL BE MIXED, SPREAD AND PLACED IN SUCH A WAY AS TO PRODUCE A UNIFORM THICKNESS OF MATERIAL AFTER PLACING. 7. EACH LAYER OF FILL SHALL BE COMPACTED WITH A MINIMUM OF 6 COMPLETE PASSES ON ALL PORTIONS OF THE SURFACE OF EACH LIFT OF FILL BY RUBBER TIRED ROLLERS, SHEEPS FOOT ROLLERS OR OTHER MECHANICAL EQUIPMENT APPROVED BY THE GEOTECHNICAL ENGINEER.
- 8. COMPACTED FILL PLACED WITHIN 4 FEET OF STRUCTURES AND PIPES SHOULD BE PLACED IN HORIZONTAL LIFTS NOT TO EXCEED 4 INCHES THICKNESS AND COMPACTED WITH HAND TAMPERS OR LIGHT COMPACTION EQUIPMENT TO THE SAME STANDARD.
- 9. HEAVY COMPACTION EQUIPMENT SHOULD NOT BE ALLOWED WITHIN 4 FEET OF STRUCTURES UNLESS A MINIMUM 2 FEET DEPTH OF FILL COVERS THE STRUCTURES. 10. WHENEVER IN PLACE DENSITIES ARE FOUND BELOW ACCEPTABLE LIMITS. ADDITIONAL ROLLING TO PRODUCE THE SPECIFIED DENSITIES SHALL BE REQUIRED.
- . THE CONTRACTOR SHALL TAKE ALL MEASURES REQUIRED TO PROVIDE FOR FREE DRAINAGE OF THE SITE AND TO PREVENT PONDING OF WATER. SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED AT ALL TIMES 12. PLACING OF FILL CONTAINING ORGANIC MATTER; PLACING OF FILL WITH MOISTURE CONTENT TOO HIGH OR TOO LOW FOR PROPER COMPACTION; PLACING OF FILL WHEN FREE WATER IS STANDING ON THE EXISTING FILL SURFACE; PLACING OF FILL IN A FROZEN CONDITION OR ON TOP OF FROZEN MATTER WILL NOT BE PERMITTED. 13. THE SOILS ENGINEER SHALL SUPERVISE THE PLACING OF THE COMPACTED FILL AND ALL THE MATERIAL AND EQUIPMENT USED FOR THIS PURPOSE AND SHALL MAKE SUCH SOILS TESTS AS MAY BE REQUIRED FOR THE COMPLETION OF THE WORK PERFORMING AT LEAST 6 IN PLACE DENSITY TESTS DURING EACH EIGHT HOUR SHIFT.
- . BOTTOM OF ALL FOOTINGS SHALL BE A MINIMUM OF 2'-O" BELOW ORIGINAL GRADE OR PLACED IN APPROVED COMPACTED FILL 2. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-6" BELOW FINISHED GRADE.
- 3. A SOIL BEARING CAPACITY OF 2000 PSF WAS USED IN THE FOUNDATION DESIGN, AND MUST BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER. IF SOIL OF THIS BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATIONS INDICATED ON THE CONTRACT DRAWINGS, FOOTINGS SHALL BE LOWERED, INCREASED IN SIZE, OR SOIL IMPROVED AS DIRECTED BY THE GEOTECHNICAL ENGINEER AND APPROVED BY MORABITO CONSULTANTS 4. EDGES OF FOOTINGS SHALL NOT BE PLACED AT A GREATER THAN 1 (VERTICAL) TO 2 (HORIZONTAL) SLOPE WITH RESPECT TO ANY ADJACENT FOOTING OR EXCAVATION.
- 5. ADJACENT COLUMN FOOTINGS THAT ABUT SHALL BE SEPARATED BY A PAPER JOINT. 6. ELEVATIONS SHOWN ON PLAN ARE TO THE BOTTOM OF THE FOOTINGS.
- . ALL CONCRETE WORK SHALL CONFORM TO ALL THE PROVISIONS OF THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301) AND TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318). 2. CONCRETE PROPERTIES FOR EACH STRUCTURAL ELEMENT IS DEFINED IN THE DESIGN DATA SECTION ON THIS SHEET. CONCRETE SHALL CONFORM TO ALL THE PROVISIONS OF "RECOMMENDED PRACTICE FOR HOT WEATHER CONCRETING" (ACI 305) AND "RECOMMENDED PRACTICE FOR COLD WEATHER CONCRETING" (ACI 306).
- 4. ALL FORMWORK SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE "FORMWORK FOR CONCRETE" SPECIAL PUBLICATION NO. 4 AND ACI'S "STANDARD RECOMMENDED PRACTICE FOR CONCRETE FORMWORK" (ACI 347). 5. CONCRETE MIX DESIGN SHALL BE BASED ON LABORATORY TRIAL BATCH METHOD DESCRIBED IN ACI-318. CONCRETE SHALL ALSO CONFORM TO THE FOLLOWING REQUIREMENTS.
- 6. ALL CONCRETE EXPOSED TO THE WEATHER SHALL HAVE AN AIR ENTRAINMENT OF 6% +/- 1%. 7. THE MAXIMUM WATER CEMENT RATIO W/C SHALL NOT EXCEED 0.56 FOR ALL CONCRETE EXCEPT CONCRETE EXPOSED TO WEATHER WHICH SHALL NOT EXCEED 0.45.
- 8. NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED. 9. THE MAXIMUM SLUMP OF ALL CONCRETE SHALL BE 4". 10. ALL CONCRETE SHALL BE CURED WITH LIQUID SEALING COMPOUND CONFORMING TO ASTM C-309, TYPE I AND FEDERAL SPECIFICATION TT-C-00800 OR OTHER APPROVED METHOD WHICH IS COMPATIBLE WITH FLOORING ADHESIVES AND OTHER SURFACE TREATMENTS. 11. ALL CONCRETE LEFT EXPOSED AT THE COMPLETION OF THE PROJECT SHALL BE TREATED WITH A CLEAR, PENETRATING ACRYLIC BASE POLYMER CAPABLE OF PREVENTING INFILTRATION OF WATER BORNE CHLORIDES SUCH AS "US CURE & SEAL" BY US CONCRETE PRODUCTS OR APPROVED EQUAL
- 12. CONTRACTOR SHALL SUPPORT ADJACENT STRUCTURES, UTILITIES, AND EXCAVATIONS AS REQUIRED FOR COMPLETION OF WORK. 13. ONE SET OF COMPRESSIVE TEST CYLINDERS FOR EACH 100 CUBIC YARDS POURED, BUT NOT LESS THAN ONE SET FOR EACH DAY'S POUR AND EACH CLASS OF CONCRETE, ALONG WITH SLUMP TESTS SHALL BE PERFORMED BY A TESTING LABORATORY APPROVED BY THE STRUCTURAL ENGINEER. 14. NO CONCRETE SHALL BE PLACED UNTIL CONCRETE DESIGN MIXES HAVE BEEN SUBMITTED FOR EACH CLASS OF CONCRETE AND HAVE BEEN APPROVED BY THE ENGINEER.
- CONCRETE ON METAL DECK: THE CONTRACTOR SHALL CALCULATE AND INCLUDE ALL ADDITIONAL CONCRETE THAT MAY BE REQUIRED DURING PLACING DUE TO DEFLECTION.
- CONCRETE SLAB ON GRADE CONSTRUCTION HE CONCRETE SLABS ON GRADE FOR THIS PROJECT HAVE BEEN DESIGNED UTILIZING A MODULUS OF SUBGRADE REACTION "K" EQUAL TO 100 PCI. 2. THE CONCRETE SLABS ON GRADE THROUGHOUT THIS PROJECT ARE NOT DESIGNED TO SUPPORT THE CRANES USED DURING THE ERECTION OF THE STRUCTURAL STEEL OR CONCRETE TILT-UP WALL BEARING PANELS IF THE CONTRACTOR ELECTS TO PLACE THE CRANE ON THE CONCRETE SLAB ON GRADE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO TAKE ALL NECESSARY PRECAUTIONS, INCLUDING THE TEMPORARY INSTALLATION OF WOOD CRIBBING ON THE SLAB, IN ORDER TO PREVENT CRACKS FROM FORMING IN THE SLABS ON GRADE. ALL CRACKS WHICH FORM IN THE CONCRETE SLABS ON GRADE DUE TO THE CRANE BEING PLACED ON THE SLAB WILL BE REPLACED OR REPAIRED TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND OWNER AT THE CONTRACTOR'S EXPENSE
- REINFORCING STEEL SHALL BE DEFORMED BARS IN ACCORDANCE WITH ASTM A 615, GRADE 60. 2. BENDS AND HOOKS ARE TO BE FABRICATED IN ACCORDANCE WITH ACI SP-66 ACI DETAILING MANUAL AND AS PER DETAILS. 3. PLACE MAIN REINFORCING STEEL SO AS TO PROVIDE 3" MINIMUM COVER FOR FOUNDATIONS POURED ON EARTH, 2" MINIMUM COVER FOR BEAMS AND COLUMNS, 3/4" MINIMUM COVER FOR SLABS AND 1 1/2" FOR ALL
- REBAR IN EXPOSED CONCRETE (EXCEPT AS OTHERWISE IN DETAILS). 4. LAP DEFORMED BARS IN ACCORDANCE WITH LAP SPLICE SCHEDULE ON THESE DRAWINGS, UNO.
- 5. HOOKS SHALL BE STANDARD HOOKS, UNO. 6. PROVIDE ACCESSORIES AND BAR SUPPORTS IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315). 7. TOP REINFORCING IN PARKING DECKS AND ALL OTHER REINFORCING SHOWN ON THE CONTRACT DOCUMENTS SHALL BE EPOXY COATED CONFORMING TO ASTM A-775 FOR BARS AND ASTM A-884 FOR WELDED WIRE
- 8. WHERE REQUIRED, DAMAGED AND CUT EPOXY COATING SHALL BE REPAIRED WITH PATCHING MATERIAL CONFORMING TO ASTM A-775. 9. EPOXY-COATED REINFORCING BARS SHALL REST ON COATED WIRE BAR SUPPORTS MADE OF DILLECTIC MATERIAL FOR A MINIMUM DISTANCE OF 2 INCHES FROM POINT OF CONTACT WITH EPOXY-COATED REBARS. 10. EPOXY-COATED REINFORCING BARS SHALL BE FASTENED WITH NYLON, EPOXY, OR PLASTIC-COATED TIE WIRE. 11. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 1064, UNO
- 12. WWF REINFORCING SHALL BE PLACED AT MID DEPTH OF SLABS ON GRADE AND DRAPED OVER SUPPORTS IN CONCRETE ON METAL DECK SLABS. 13. END LAPS OF ALL WWF REINFORCING SHALL BE LAPPED (1) SQUARE
- 14. CONCRETE ENGINEERED REINFORCING FIBERS SHALL BE POLYPROPYLENE, COLLATED, FIBRILLATED FIBERS FROM FIBERMESH, INC. INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. 15. GLASS FIBER REINFORCED POLYMER (GRRP) BARS SHALL BE GRADE III MEETING ALL THE REQUIREMENTS OF ASTM D8505. ALL BARS SHALL HAVE DEFORMED RIBS AND BOND STRENGTH PER ASTM D7913. MINIMUM TENSILE STRENGTH SHALL BE 145 KSI FOR ALL BARS.
- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. 2. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING
- THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP. FATIGUE. IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE 3. ANCHOR CAPACITY IS HIGHLY DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE/MASONRY. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS. IF EDGE DISTANCES OR ANCHOR SPACING IS NOT SPECIFIED ON THE DRAWINGS, PROVIDE THE FOLLOWING MINIMUM DISTANCES. A. EDGE DISTANCES a. ADHESIVE ANCHORS: 2 TIMES THE ANCHOR EMBEDMENT LENGTH
- b. UNDERCUT ANCHORS: 2.5 TIMES THE ANCHOR EMBEDMENT LENGTH c. EXPANSION ANCHORS (SLEEVE OR WEDGE): 4 TIMES THE ANCHOR EMBEDMENT LENGTH B. ANCHOR SPACING
- ALL ANCHORS: 3 TIMES THE ANCHOR EMBEDMENT 4. ANCHORS SHALL BE INSTALLED BY QUALIFIED PERSONNEL IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS, BUILDING CODE, AND MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). 5. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL POST-INSTALLED ANCHORS HAVE BEEN PROPERLY TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING FOR EACH SPECIFIC PRODUCT 6. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS (AS DETERMINED BY THE ENGINEER) SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE
- ACI-CRSI "ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM". 2. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFICIAL. THE SPECIAL INSPECTOR SHALL FURNISH A REPORT TO THE STRUCTURAL ENGINEER OF RECORD AND BUILDING OFFICIAL THAT THE WORK COVERED BY THE REPORT HAS BEEN PROPERLY PERFORMED AND THAT THE MATERIALS USED AND THE INSTALLATION PROCEDURES USED CONFORM WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).
- 8. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. IF HIGH-EARLY STRENGTH CONCRETE MIXES ARE SPECIFIED. CONTACT THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL OF MINIMUM INSTALLATION AGE. 9. EXISTING REINFORCING BARS OR PRESTRESSING STEEL IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE
- CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TESTING TO LOCATE THE POSITION AND DEPTH OF THE REINFORCING BARS OR PRESTRESSING AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY HILTI FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS. 10. EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES. ANCHORS EXPOSED TO WEATHER AND AT SILL PLATES SHALL BE STAINLESS STEEL. A. MECHANICAL ANCHORS IN CRACKED OR UNCRACKED CONCRETE USE:
- a. HILTI KWIK BOLT-TZ2 EXPANSION ANCHORS b. HILTI KWIK HUS-EZ AND KH-EZ I SCREW ANCHORS B. ADHESIVE ANCHORS IN CRACKED AND UNCRACKED CONCRETE USE:
- a. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HIT-Z ROD b. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM WITH HAS-E THREADED ROD C. REBAR DOWELING INTO CONCRETE:
- a. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM D. ADHESIVE ANCHORS IN HOLLOW / GROUTED / MULTI-WYTHE MASONRY USE:
- a. HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM E. MECHANICAL ANCHORS IN GROUTED MASONRY USE: a. HILTI KWIK BOLT-1 EXPANSION ANCHORS
- b. HILTI KWIK HUS-EZ AND KH-EZ I SCREW ANCHORS

(Fy = 50 KSI) FOR PAINTED DECK UNITS. SLAB FORM DECKS SHALL HAVE Fy = 60 KSI. 3. GALVANIZED UNITS SHALL CONFORM TO ASTM A-924, COATING CLASS G-60 FOR INTERIOR FLOOR DECK AND CLASS G-90 FOR EXTERIOR FLOOR DECK AND ALL ROOF DECK. 4. PAINTED UNITS SHALL BE A BAKED-ON RUST INHIBITIVE PAINT COMPRISING OF A 2 COAT SYSTEM OF CHROMATE PRIMER AND ACRYLIC TOPCOAT. 5. ALL UNITS SHALL SPAN CONTINUOUS OVER A MINIMUM OF 3 SUPPORTS. 6. ALL METAL DECK UNITS SHALL BE WELDED TO THE STEEL FRAMING AT 12" o/c MAXIMUM UNLESS SPECIFIED AT A CLOSER SPACING BY THE MANUFACTURER. 7. METAL DECK AND/OR SLAB FORM WITH A THICKNESS LESS THAN 22 GAUGE SHALL BE WELDED WITH WELDING WASHERS. 8. ALL SIDE LAPS SHALL BE CONNECTED USING SHEET METAL SCREWS OR WELDS AT 24" o/c MAXIMUM. 9. ALL WELDS AND BURNED AREAS SHALL BE THOROUGHLY CLEANED AND PAINTED WITH AN APPROVED PRIMER. 10. WHEN ROOF DECK SLOPE IS PERPENDICULAR TO SUPPORT MEMBERS (BEAM, JOISTS, TRUSS, ETC.) SPAN PROVIDE CONTINUOUS BENT PLATE OR STEEL SHIMS BY STRUCTURAL STEEL FABRICATOR TO ASSURE FULL DECK BEARING AT SUPPORT. 11. NO CONDUIT SHALL BE PLACED IN CONCRETE SLAB ON METAL DECK. 1. REINFORCED MASONRY CONSTRUCTION SHALL CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, ACI 530, AND SPECIFICATIONS FOR MASONRY STRUCTURE, ACI 530. 2. MASONRY SHALL BE IN ACCORDANCE WITH ASTM C 90. 3. MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI BASED ON THE NET CROSS SECTIONAL AREA OF THE INDIVIDUAL BLOCK UNITS. PROVIDE HIGHER UNIT STRENGTHS WHERE REQUIRED BY DRAWINGS. 4. ALL MORTAR SHALL BE TYPE "S" "MORTAR CEMENT" OR "PORTLAND CEMENT/LIME" CONFORMING TO ASTM C 270. 5. MASONRY REINFORCING SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. 6. GROUT SHALL CONFORM TO ASTM C476, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI. 7. REINFORCED VOIDS IN MASONRY SHALL BE FILLED SOLID WITH GROUT. 8. PROVIDE A MINIMUM OF 3 COURSES OF SOLID BRICK OR ONE COURSE OF 100% SOLID BLOCK UNDER WALL BEARING ENDS OF ALL JOISTS AND SLABS, THE FULL WIDTH OF THE WALL, UNLESS NOTED. 9. IN BEARING WALLS, PROVIDE SOLID BRICK OR 100% BLOCK EXTENDING 8" BEYOND WALL OPENINGS THE FULL WALL THICKNESS DOWN TO THE FLOOR, UNLESS NOTED. 10. ALL PORTIONS OF MASONRY WALLS HAVING A HORIZONTAL CROSS SECTION OF 4 SQ. FT. OR LESS SHALL BE OF SOLID MASONRY DOWN TO FOOTINGS. 11. PROVIDE 9 GAUGE LADDER TYPE HORIZONTAL JOINT REINFORCING AT 16" 0/c IN ALL MASONRY WALLS UNLESS NOTED OTHERWISE. 12. ALL MASONRY WALLS SHALL HAVE CONTROL JOINTS AT 40'-0" o/c MAXIMUM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS. 13. ALL MORTAR JOINTS IN MASONRY WALLS (HORIZONTAL AND VERTICAL) SHALL BE FILLED 100% WITH MORTAR. 1. THE METAL STUD TRUSSES SHALL BE CUSTOM DESIGNED TO FIT THE DIMENSIONS AND LOADS INDICATED ON THE PLANS. 2. ALL DESIGNS SHALL BE IN ACCORDANCE WITH THE ALLOWABLE LOAD VALUES SHOWN ON THESE DRAWINGS. 3. ALL PREFABRICATED TRUSSES SHALL HAVE ERECTION BRACING, STRUT BRACING AND BRIDGING AS REQUIRED BY THE MANUFACTURER TO RESIST ALL CONSTRUCTION AND BUILDING LOADS. 4. SHOP DRAWINGS INCLUDING DESIGN CALCULATIONS, MEMBER FORCES AND STRESS CONTROL POINTS SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF THE PROJECT AND SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL.

2. STEEL ROOF & COMPOSITE DECK AND ALL ACCESSORIES SHALL BE FORMED OF ZINC COATED STEEL SHEETS CONFORMING TO ASTM A-653, SQ (Fy = 50 KSI) FOR GALVANIZED DECK UNITS OR ASTM A-1008

1. ALL METAL DECKING SHALL BE DESIGNED, FABRICATED, AND ERECTED PER THE "SPECIFICATION OF THE STEEL DECK INSTITUTE", EXCEPT AS NOTED HEREIN.

- 1. MASONRY ANCHORS FOR BRICK VENEER AND STRUCTURAL METAL STUD EXTERIOR WALLS SHALL BE THE FOLLOWING: POS.I.TIE BY HECKMANN BUILDING PRODUCTS, INC, UNO. 2. ALL MASONRY ANCHORS SHALL BE ATTACHED THRU THE GYPSUM SHEATHING TO THE STEEL STUDS WITH ONE SELF DRILLING, SELF TAPPING SCREWS. 3. ALL TIES MUST BE EMBEDDED AT LEAST 2 IN. INTO THE BED JOINTS OF THE BRICK VENEER. THEY MUST BE SECURELY ATTACHED TO THE STEEL STUDS THROUGH THE SHEATHING, AND NOT TO THE
- SHEATHING ALONE. 4. ADDITIONAL TIES SHOULD BE INSTALLED AT APPROXIMATELY 8" o/c AT JAMBS AND NEAR EDGES. 5. THE ANCHORS SHALL BE SPACED EQUALLY THROUGHOUT THE PANEL WITH ONE TIE EVERY 2 SQUARE FEET.
- 6. THE TIES SHALL BE MADE OF 3/16 INCH DIAMETER WIRE IN A TRIANGULAR SHAPE. 7. IT IS MANDATORY THAT THE WIRE TIE EXPERIENCES NO FREE HORIZONTAL MOVEMENT PERPENDICULAR TO THE STUD AND BRICK THUS GUARANTEEING THAT ALL LATERAL LOADS ARE TRANSFERRED IMMEDIATELY TO THE METAL STUD BACKUP.
- 8. ALL COMPONENTS OF THE MASONRY ANCHOR SYSTEM SHALL BE GALVANIZED. 9. WHERE STUD SPACING IS REDUCED TO LESS THAN 16" o/c DUE TO LATERAL LOADS, PROVIDE ADDITIONAL MASONRY ANCHORS AT EVERY STUD.
- 1. ALL OPENINGS IN NEW WALLS AND PARTITIONS ARE TO BE PROVIDED WITH LINTELS. 2. LINTELS SHALL BE STONE, CONCRETE, SLAG CONCRETE, OR STRUCTURAL STEEL.
- 3. PROVIDE 4" MINIMUM END BEARING FOR LINTELS IN NON BEARING PARTITIONS AND 8" MINIMUM END BEARING FOR LINTELS IN ALL EXTERIOR WALLS AND BEARING PARTITIONS. 4. FOR ANY OPENING NOT SPECIFICALLY SHOWN, PROVIDE A. ONE 4" x 3 1/2" x 5/16" (LLV) ANGLE FOR EACH 4" OF WALL THICKNESS FOR SPANS NOT EXCEEDING 6'-0" B. ONE 6" x 3 1/2" x 5/16" (LLV) ANGLE FOR EACH 4" OF WALL THICKNESS FOR SPANS EXCEEDING 6'-0" BUT LESS THAN 8'-0" OR PRECAST CONCRETE LINTELS AS DIRECTED BY THE ARCHITECT.
- PRECAST CONCRETE LINTELS SHALL HAVE: A. ONE #4 TOP AND BOTTOM FOR EACH 4" OF WALL THICKNESS FOR SPANS NOT EXCEEDING 6'-0"
- B. ONE #5 TOP AND BOTTOM FOR EACH 4" OF WALL THICKNESS FOR SPANS EXCEEDING 6'-0" BUT LESS THAN 8'-0". 6. ALL PRECAST CONCRETE LINTELS SHALL ALSO BE REINFORCED WITH #2 WIRE TIES AT 8"o/c.
- 7. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND STRUCTURAL DRAWINGS FOR LOCATIONS OF LINTELS. 8. CONSULT STRUCTURAL ENGINEER FOR LINTEL REQUIREMENTS FOR ALL NEW OPENINGS IN EXISTING WALLS.
- . METAL STAIR STEEL FRAMING SHOWN ON THE STRUCTURAL DRAWINGS IS DIAGRAMMATIC AND INTENDED TO ESTABLISH MINIMUM MEMBER SIZES AND PROFILES. 2. FABRICATOR IS RESPONSIBLE FOR DESIGNING SYSTEM AND SHALL EMPLOY A REGISTERED STRUCTURAL ENGINEER TO ENGINEER EACH COMPONENT OF STAIR SYSTEM. EXPOSED STEEL SHALL COMPLY WITH THE APPEARANCE AND SURFACE PREPARATION REQUIREMENTS OF ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS).
- 4. REFER TO ARCHITECTURAL DRAWINGS FOR A MORE DETAILED DOCUMENTATION OF THE STAIR REQUIREMENTS. 1. SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED BY GENERAL CONTRACTOR AND REVIEWED BY THE ENGINEER.
- 2. ALL SHOP DRAWINGS USED FOR WORK SHALL BEAR THE STAMP OF THE ARCHITECT/ENGINEER AND SHALL BE MARKED "APPROVED" OR "APPROVED AS NOTED" 3. CONTRACTOR SHALL CHECK SHOP DRAWINGS THOROUGHLY BEFORE SUBMITTING. VERIFY DIMENSIONS REQUIRING FIELD VERIFICATION BEFORE SUBMITTING AND MARK AS HAVING BEEN VERIFIED. 4. SUBMIT METAL DECK SHOP DRAWINGS 5. SUBMIT CONCRETE MIX DESIGNS FOR EACH CLASS OR USE
- 6. SUBMIT CONCRETE REINFORCING STEEL SHOP DRAWINGS 7. SUBMIT MASONRY REINFORCING STEEL SHOP DRAWINGS.
- SUBMIT LIGHT GAUGE TRUSS FRAMING SHOP DRAWINGS. 9. ALL CONTRACTOR MODIFICATIONS (INCLUDING PRODUCTS SUBMISSION) MUST BE IDENTIFIED IN WRITING AS A PROPOSED "AS EQUAL" CHANGES AT TIME OF SUBMISSION. 10. IF A CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS OR FAILS TO FOLLOW THE ABOVE "AS EQUAL" PROCEDURE, THE FIRM MORABITO CONSULTANTS, INC. WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT. 11. SHOP DRAWINGS ARE REVIEWED BY THE ENGINEER AS A CONVENIENCE TO THE CONTRACTOR AND ARE NOT A CONTRACT DOCUMENT.

DELEGATED DESIGN ITEM 1. CONTRACTOR SHALL RETAIN A QUALIFIED SPECIALTY ENGINEER LICENSED IN THE PROJECT JURISDICTION TO DESIGN AND DETAIL PERFORMANCE SPECIFIED COMPONENTS, INCLUDING BUT NOT LIMITED

- a. METAL STAIRS b. RAILINGS AND GUARDRAILS
- c. LIGHT GAUGE TRUSSES d. STOREFRONTS
- e. PRECAST CONCRETE f. SHORING / SCAFFOLDIN
- g. CONCRETE UNDERPINNING 2. SHOP DRAWINGS AND CALCULATIONS SHALL BEAR THE SEAL AND SIGNATURE OF THE PROFESSIONAL ENGINEER.

OF THE PLANS AND SPECIFICATIONS WITHOUT THE WRITTEN AUTHORIZATION OF MORABITO CONSULTANTS, INC.

- 3. WHERE ANY COMPONENT ATTACHES TO THE BASE BUILDING, SUBMIT A COMPLETE DESCRIPTION OF ALL LOADS TRANSFERRED TO THE BASE BUILDING STRUCTURE. THIS INCLUDES MAGNITUDE OF THE LOADS AS WELL AS DESCRIPTIONS OF THE TYPE OF CONNECTION USED AT EACH POINT OF CONTACT. 4. ALL DELEGATED DESIGN SUBMITTALS SHALL BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO COMMENCING FABRICATION OR ERECTION/INSTALLATION, AND APPROVED COPIES OF ALL DELEGATED DESIGN SUBMITTALS SHALL BE AVAILABLE ON SITE FOR INSPECTOR REVIEW.
- ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE BUILDING CODE AND ALL LOCAL ORDINANCES. 2. THE OWNER SHALL HIRE AN EXPERIENCED QUALIFIED INSPECTOR TO PERFORM ALL REQUIRED INSPECTION WORK.
- 3. INSPECTION SHALL CONSIST OF VISUAL OBSERVATIONS OF MATERIALS, EQUIPMENT OR CONSTRUCTION WORK FOR THE PURPOSE OF ASCERTAINING THAT THE WORK IS IN SUBSTANTIAL CONFORMANCE WITH THE CONTRACT DOCUMENTS AND WITH THE DESIGN INTENT. 4. THE ENGINEER WILL NOT PERFORM THE REQUIRED INSPECTION AS PART OF THIS PRESENT CONTRACT WITH THE ARCHITECT/OWNER. HOWEVER, IF DESIRED, MORABITO CONSULTANTS, INC. MAY BE
- HIRED UNDER A SEPARATE CONTRACT TO PERFORM THIS INSPECTION WORK. 5. UNDER THIS PRESENT CONTRACT, THE ENGINEER MAY PERFORM CONSTRUCTION ADMINISTRATION SERVICES OR VISIT THE SITE TO ASCERTAIN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS. HOWEVER, SUCH SERVICES OR SITE VISITS SHALL NOT BE RELIED UPON BY OTHERS AS INSPECTION OR ACCEPTANCE OF THE WORK, NOR SHOULD IT BE CONSTRUED TO RELIEVE THE
- CONTRACTOR IN ANY WAY FROM HIS OBLIGATIONS AND RESPONSIBILITIES UNDER THE CONSTRUCTION CONTRACT 6. IT IS AGREED THAT IF THE OWNER DOES NOT ENGAGE MORABITO CONSULTANTS OR AN INDEPENDENT THIRD PARTY INSPECTION AGENCY. THEN THE OWNER WILL DEFEND. INDEMNIFY AND HOLD HARMLESS MORABITO CONSULTANTS, INC., FROM ANY CLAIM OR SUIT WHATSOEVER, INCLUDING BUT NOT LIMITED TO ALL PAYMENTS, EXPENSES OR COSTS INVOLVED, ARISING FROM OR ALLEGED TO HAVE ARISEN FROM THE CONTRACTOR'S PERFORMANCE OR THE FAILURE OF THE CONTRACTOR'S WORK TO CONFORM TO THE DESIGN INTENT AND THE CONTRACT DOCUMENTS.
- 7. MORABITO CONSULTANTS, INC., AGREES TO BE RESPONSIBLE FOR ITS OWN OR ITS EMPLOYEES' NEGLIGENT ACTS, ERRORS OR OMISSIONS. EXISTING CONDITIONS . ALL DIMENSIONS AND ELEVATIONS OF EXISTING STRUCTURES SHOWN ON THE DRAWINGS ARE OBTAINED FROM AVAILABLE SOURCES, AND ARE NOT GUARANTEED TO BE TRUE AND EXACT. THE GENERAL CONTRACTOR SHALL VERIFY THESE DIMENSIONS AND ELEVATIONS BY ACTUAL FIELD MEASUREMENTS PRIOR TO FABRICATION OF ANY MATERIALS AND START OF ANY WORK, AND REPORT ANY
- DISCREPANCIES TO THE ARCHITECT/ENGINEER. 2. FOR ADDITIONAL INFORMATION ON THE EXISTING CONSTRUCTION, THE CONTRACTOR SHALL REFER TO DRAWINGS OF THE EXISTING STRUCTURE. 3. IN AS MUCH AS THE REMODELING AND/OR REHABILITATION OF AN EXISTING BUILDING REQUIRES THAT CERTAIN ASSUMPTIONS BE MADE REGARDING EXISTING CONDITIONS, AND BECAUSE SOME OF THESE ASSUMPTIONS CANNOT BE VERIFIED WITHOUT EXPENDING GREAT SUMS OF ADDITIONAL MONEY, OR DESTROYING OTHERWISE ADEQUATE OR SERVICEABLE PORTIONS OF THE BUILDING, THE OWNER AGREES THAT. EXCEPT FOR NEGLIGENCE ON THE PART OF MORABITO CONSULTANTS, THE OWNER WILL HOLD HARMLESS AND INDEMNIFY MORABITO CONSULTANTS FOR AND AGAINST ANY AND ALL CLAIMS, DAMAGES, AWARDS, AND COSTS OF DEFENSE ARISING OUT OF DEFICIENCIES IN THE ORIGINAL BUILDING STRUCTURE.

3. THE CONTRACTOR AGREES TO HOLD HARMLESS AND INDEMNIFY MORABITO CONSULTANTS, INC., AGAINST ALL DAMAGES, CLAIMS, AND LOSSES, INCLUDING DEFENSE COSTS, ARISING OUT OF ANY REUSE

STEEL PLATES & BARS ASTM A-36 Fy = 36,000 PSI, UNO <u>LIVE LOADS</u>: SLAB ON GRADE = 150 PSF ROOF = 30 PSF + SNOW DRIFT ROOF SNOW LOAD 1. GROUND SNOW LOAD: Pg = 25 PSF 2. SNOW EXPOSURE FACTOR: Ce = 0.9 3. IMPORTANCE FACTOR: Is = 1.10

REINFORCING STEEL: Fy = 60,000 PSI (UNO)

DESIGN DATA

STRENGTH (f'c) AS FOLLOWS:

SLAB ON GRADE

FOOTINGS

STRUCTURAL STEEL: ANGLES

STEEL PIPE

6

ALL CONCRETE, UNO

BUILDING CODE: THE INTERNATIONAL BUILDING CODE - IBC 2018

EXTERIOR CONCRETE 4500 PSI

NORMAL WEIGHT CONCRETE HAVING A MINIMUM 28 DAY COMPRESSIVE

3000 PSI

3000 PSI

3000 PSI

ASTM A-36 Fy = 36,000 PSI, UNO

ASTM A-53 GR. B Fy = 35,000 PSI, UNO

THERMAL FACTOR: Ct = 1.0 5. FLAT-ROOF SNOW LOAD: Pf = 17.33 PSF 6. BALANCED SNOW LOAD: 13.1 PSF 7. UNBALANCED SNOW LOAD: 28.1 PSF

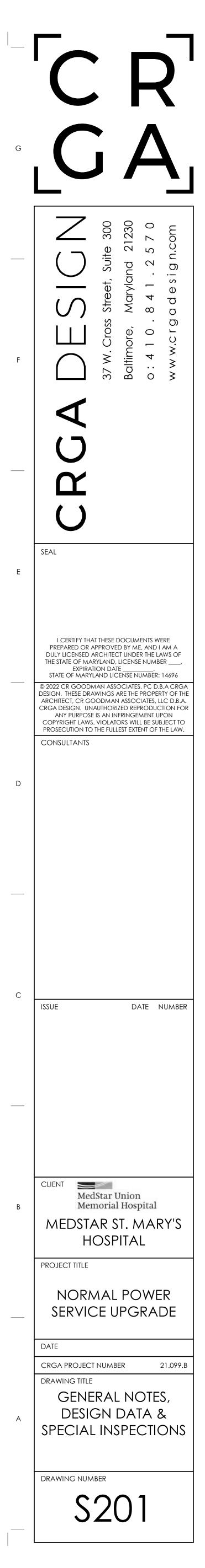
. ULTIMATE WIND SPEED: Vult = 122 MPH RISK CATEGORY III

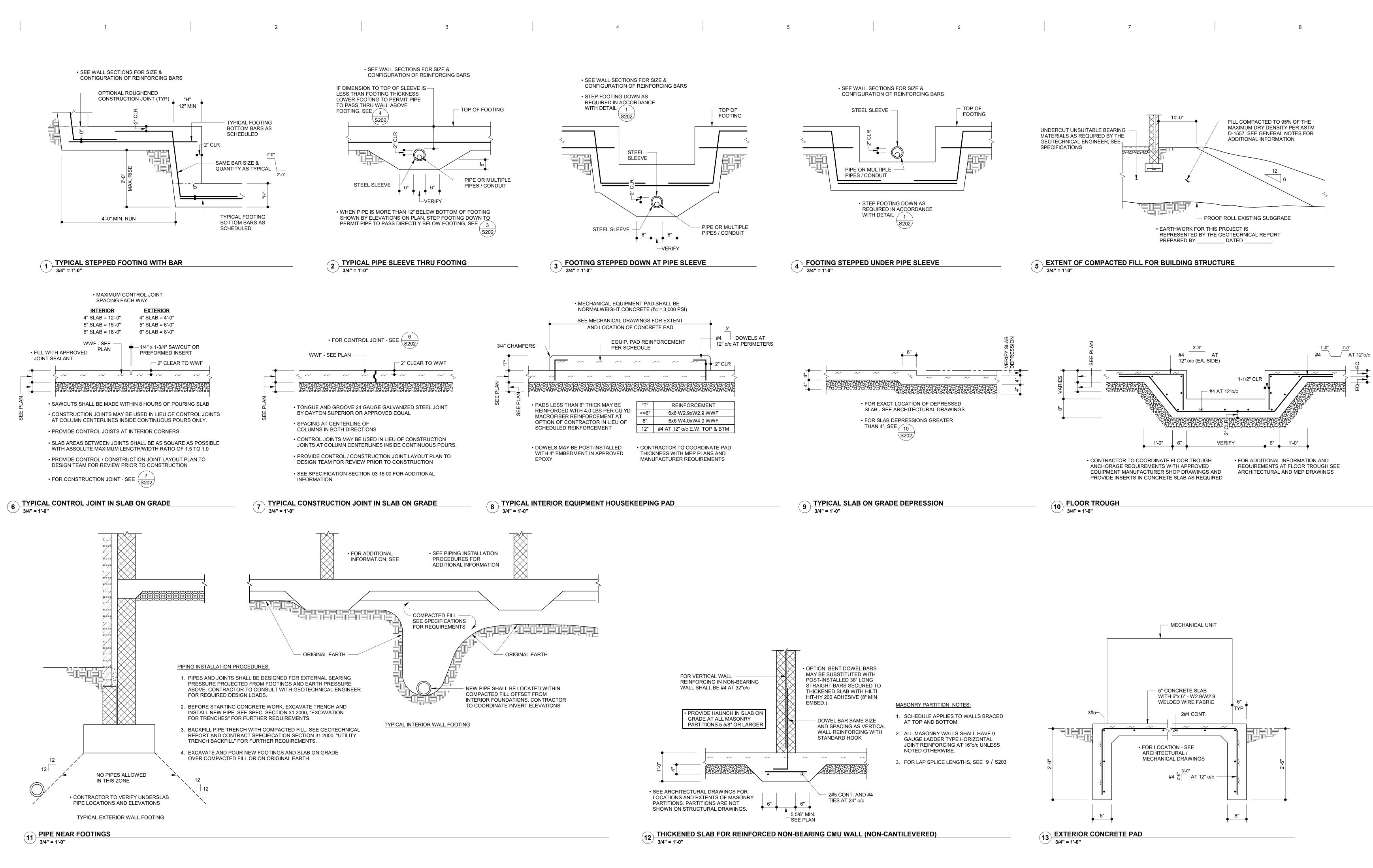
2. NEVERTHELESS, THE PLANS AND SPECIFICATIONS PREPARED UNDER THIS AGREEMENT SHALL REMAIN THE PROPERTY OF MORABITO CONSULTANTS, INC. UPON COMPLETION OF THE WORK.

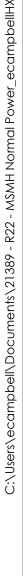
- 3. EXPOSURE CATEGORY B 4. INTERNAL PRESSURE COEFFICIENT = ±0.18 5 DIRECTIONALITY Kd = 0.85
- 6. COMPONENTS AND CLADDING SEE DIAGRAM/TABLE ON S301
- EARTHQUAKE LOAD 1. RISK CATEGORY = III

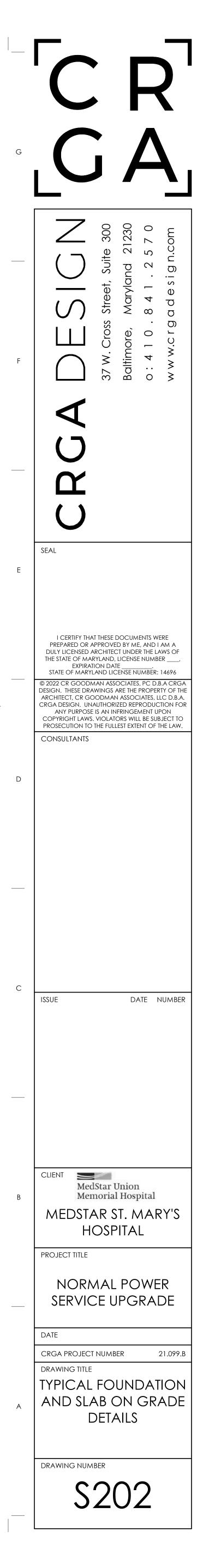
THE CONTRACTOR ACKNOWLEDGES THESE PLANS AND SPECIFICATIONS PREPARED BY MORABITO CONSULTANTS. INC., AS INSTRUMENTS OF PROFESSIONAL SERVICE.

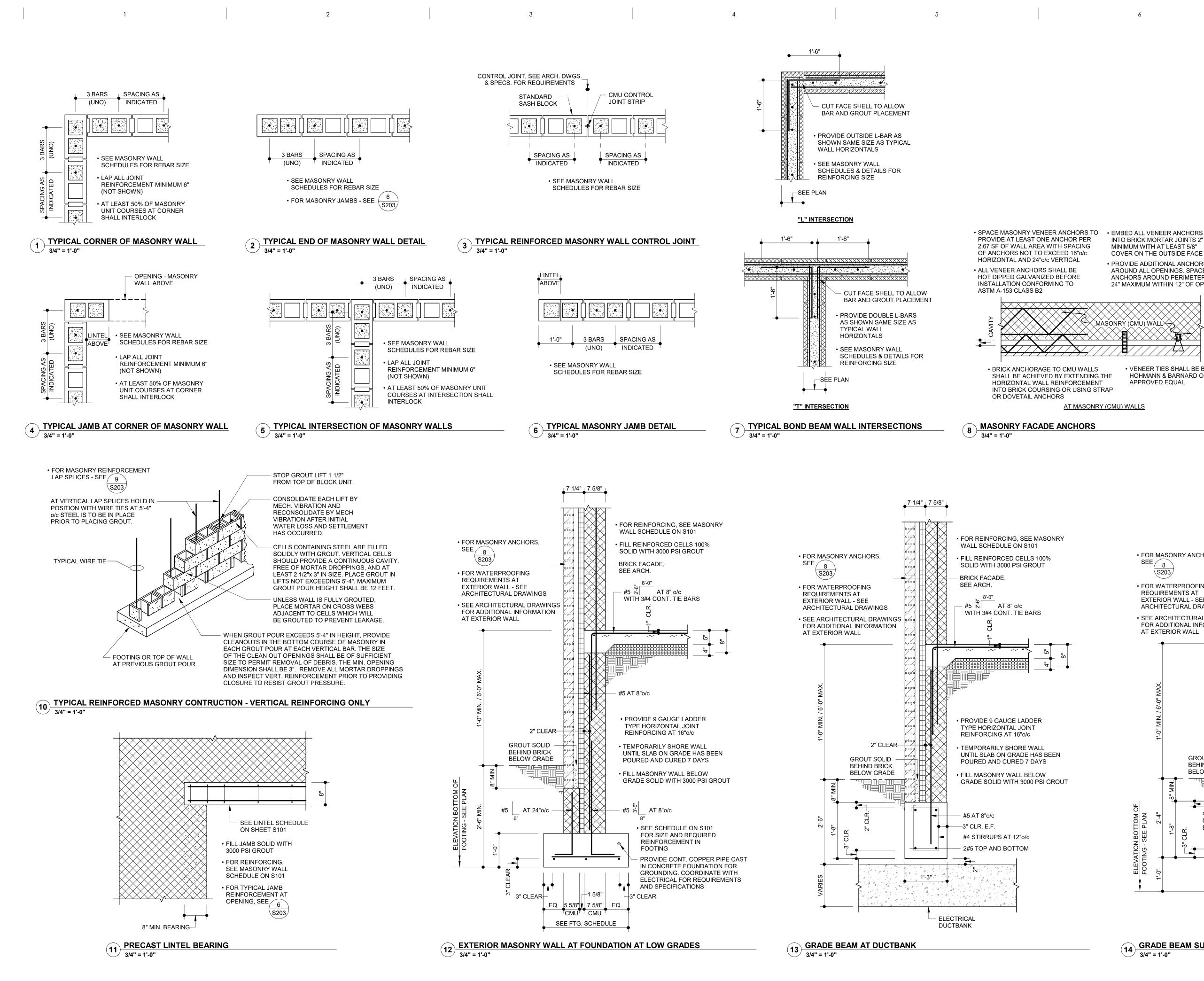
- 2. IMPORTANCE FACTOR: le = 1.25 SITE CLASS D 4. MAPPED SPECTRAL RESPONSE ACCELERATION
- a. S_S≤0.122 b. S₁ ≤ 0.042 5. SPECTRAL RESPONSE COEFFICIENTS
- a. S_{DS} = 0.130 b. S_{D1} = 0.067
- 6. SEISMIC DESIGN CATEGORY B 7. SEISMIC FORCE-RESISTING SYSTEM: ORDINARY REINFORCED MASONRY SHEAR WALLS
- 8. DESIGN BASE SHEAR: V = 4.8 KIPS 9. SEISMIC RESPONSE COEFFICIENT: Cs = 0.0813
- 10. RESPONSE MODIFICATION FACTOR: R = 2.0 11. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE DESIGN METHOD





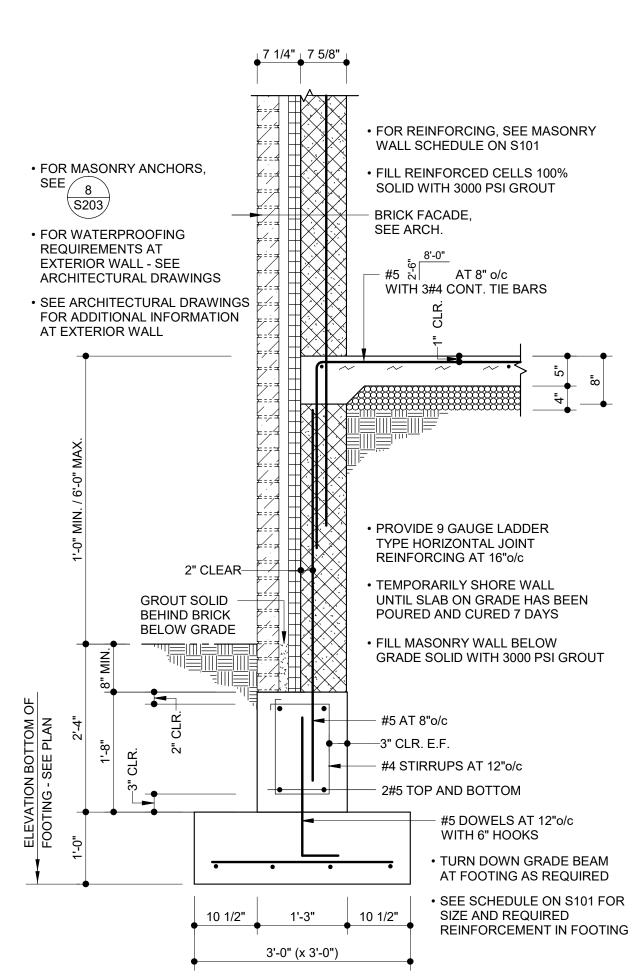






1 5 5 8 8

(14) GRADE BEAM SUPPORT AT DUCTBANK 3/4" = 1'-0"



VENEER TIES SHALL BE BY HOHMANN & BARNARD OR APPROVED EQUAL

MINIMUM WITH AT LEAST 5/8" COVER ON THE OUTSIDE FACE PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS. SPACE ANCHORS AROUND PERIMETER AT 24" MAXIMUM WITHIN 12" OF OPENING

INTO BRICK MORTAR JOINTS 2"

6

NORMAL BLOCK 8" 2 7/8" 1 BAR PER CELL 10" ──┐┌──╤╱╌╱╗┟╝╏╷┌────┐┌─ - 11 **6** 6 5 1 12"

1. REINFORCEMENT LAPS SHALL OCCUR ABOVE THE FLOOR LINE, WHERE LARGER BAR SIZE SHALL EXTEND FROM WALL BELOW. 2. TOLERANCES FOR PLACEMENT OF VERTICAL REINFORCING IS +/-

- 1/2 INCH.
- 2 BAR PER CELL
- OTHERWISE ON DETAILS.

- 3. FOLLOW TABLE FOR EDGE DISTANCES UNLESS NOTED

4. SECURE REINFORCING IN PLATE TO PREVENT DISPLACEMENT

BY CONSTRUCTION LOADS OR BY PLACEMENT OF GROUT.

6. FOR 6" BLOCK, JOINT MORTAR FINS SHALL BE REMOVED FROM

REINFORCEMENT SPLICES TO MINIMIZE CONGESTION OF

BY LENTON OR APPROVED EQUAL AND DEVELOP 125% OF

8. LAP LENGTH VALUES IN BOLD ARE CONTROLLED BY MINIMUM 48

MASONRY WALLS UNDER CONSTRUCTION". WALLS GROUT

SHALL BE REQUIRED TO CURE FOR MINIMUM OF 24 HOURS.

BAR DIAMETERS PER THE "STANDARD PRACTICE FOR BRACING

TYPICAL MASONRY WALL REINFORCEMENT LAP SCHEDULE & PLACEMENT

VERTICAL BARS. MECHANICAL COUPLERS SHALL BE FABRICATED

5. REBAR LAP SPLICES AS NOTED IN SCHEDULE.

CELLS FOR PLACEMENT OF #5 OR #6 BARS.

REINFORCEMENT STRENGTH.

[/] 3/4" = 1'-0"

7. CONTRACTOR MAY ELECT TO PROVIDE MECHANICAL

- MASONRY REBAR LAP SPLICES BLOCK 6" 8" OR 10" SIZE 1 BAR BAR PER SIZE CELL 18" 18" #3
 - 1 BAR | 2 BAR | 1 BAR | 2 BAR PER PER PER PER CELL CELL CELL CELL 18" 18" 18" 24" 24" 24" 24" #4 24" 30" 30" 30" 30" 30" #5 53" 37" 57" 36" 57" #6 52" #7 42" 80" 50" #8 --64"

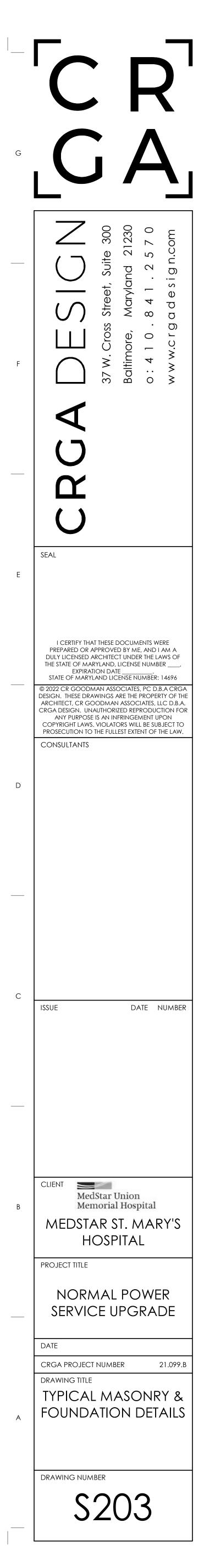
12" OR

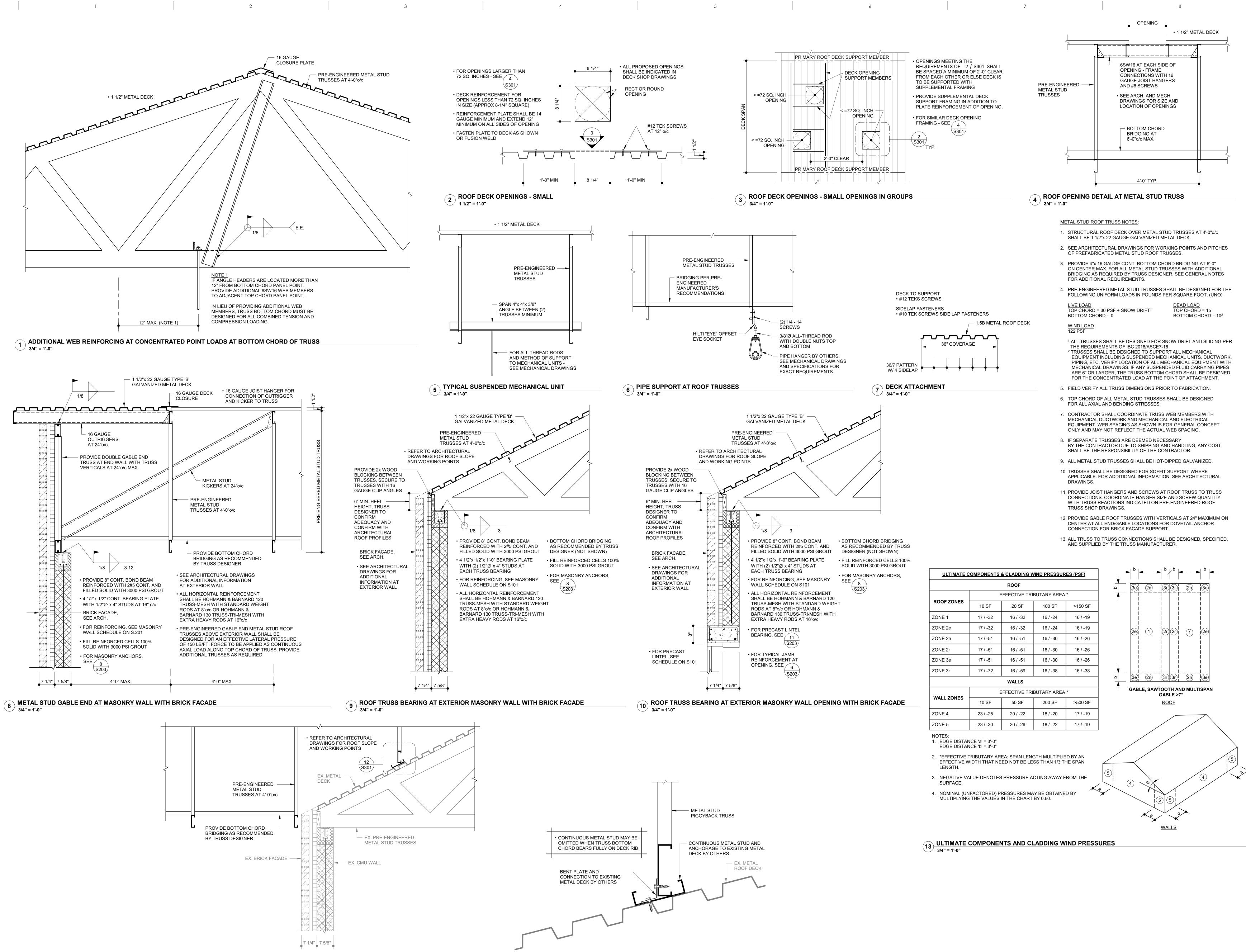
LARGER

8

VALUES IN BOLD ARE CONTROLLED BY MINIMUM 48 BAR DIAMETERS PER THE "STANDARD PRACTICE FOR BRACING MASONRY WALLS UNDER CONSTRUCTION"

7





(11) NEW ROOF TRUSS TRANSITION TO PIGGYBACK TRUSSES ON EXISTING ROOF FRAMING / 3/4" = 1'-0"

1

12 PIGGYBACK TRUSS BEARING AT ROOF DECK 3" = 1'-0"

6

7

8

